



Conduct appraisal of steel fiber reinforced concrete beams in terms of flexion and shear

S. Kamalkannan¹, S. Thowfica²

^{1,2}Assistant Professor, Department of Civil Engineerig,

Surya engineering college, Mettukadai

Email: kannanapr20@gmail.com

Email: taufiq.1000@gmail.com

Abstract

This An investigational research included continuing on through the direct of shafts under flexure. These days perilous occasion like shiver, wind power, etc expects a colossal part in the improvement business. So structures and other improvement work should be coordinated in agreeable manner, which struggle with higher burdens and seismic forces. Versatility and energy osmosis limit are the basic central of the shake safe new development. Fiber kept up strong social affairs high strength, improved pliability and redesigning energy upkeep limit. In this fiber is melded various degrees to extend the strength of concrete. In this we add various admixtures like silica rage as a cementitious material to secure strength when it is mixed in with water and cerahyper plasizicer as a substance admixture to diminishing the water solid degree and extending the value of the strong. By then this paper presents the eventual outcomes of a nonlinear Finite Element (FE) evaluation drove on Reinforced High Performance Concrete (HPC). Showing the conflicting lead of looked after strong, which is both non-homogeneous and isotropic, is an aggravating go against in the bound part evaluation of fundamental orchestrating enhancements.

Keywords: flexure, fiber looked after strong, flexibility, water solid degree

I. Introduction

Concrete is an essential fragments exist in advancements and expansions in different plans. Understanding the response of these parts during stacking is crucial for the improvement of an as a rule beneficial and safe development. Different procedures have been utilized to look at the response of central parts. Test based testing has been everything viewed as used as an approach to manage see explicit bits and the effects of strong strength under stacking. While this is a strategy that produces authentic response, it is extraordinarily drawn-out and the use of materials can be especially outrageous. The utilization of restricted area evaluation to consider these parts has likewise been used. Of late, regardless, the use of restricted part evaluation has expanded because of moving data and cutoff points of PC programming and stuff. It has now become the choice structure to confine solid basic territories. The use of program to show these parts is much faster, and unfathomably sensible. The usage of FEA has been the maintained strategy to consider the direct of concrete. This increase work contains spaces of study, for instance, Behavior at First Cracking, Behavior at Initial Cracking, Behavior past First Cracking, ,Load-Deformation Response of control section and Application of Effective Prestress, Self-Weight, Zero Deflection, Decompression, Initial Cracking, Secondary Linear Region, Behavior of Steel Yielding and Beyond, Flexural Limit State of prestressed strong bar . The monograph contains commitments that chart uses of the bound part framework for taking apart post-top cyclic direct and versatility of kept up strong portions. The objective of this paper was to examine and survey the utilization of the restricted segment methodology for the evaluation of made strong sections. Most importantly, shaping review was worked with to survey past test and quick systems related to looked after concrete.

The agreement relied upon kept up strong area direct from the start breaking, lead past first breaking, direct of help yielding and past, strength limit state, load-distortion response, and break plan. The results got was twirled around like before additionally assessment of first breaking load, crazy weight, work-a done in straight and nonlinear territory, and weight redirection nature between these undeniable help level of the reliable shaft.

Giuseppe Campione.et.al: The flexural direct of plain and wiry made cement (FRC) sends under monotonic and cyclic exercises was examined. The most conspicuous fibers utilized are gotten steel ones, and the best rate for crucial application is some spot in the degree of 0.5% and 1.5% by volume of concrete. The test outcomes got show that the use of fibers, in a correct rate for chief purposes, and in blend in with standard steel support, allows the achievement of better shows stood separated from those of regularly made transmits.

H. Akbarzadeh. et.al: This paper presents a groundwork program impelled consider the flexural direct and redistribution in depiction of kept up high strength concrete (RHSC) tenacious segments created with CFRP and GFRP sheets. Five gigantic degree reliable (two-space) sends (150 _ 250 _ 6000 mm) were tried to frustration on a control fragment and four RHSC radiates fortified with distantly created CFRP and GFRP sheets on the strong pressing variable countenances. Relationship among principal and expected second and weight limit show that the proposed model agrees very well with the test results, as such guarding the usage of the proposed model for HSC and NSC in created bars.

Samir A. et.al: In this journal they were study the effects of steel strands, longitudinal pliant help degree and concrete compressive strength on the flexural lead of made strong shafts. 27 fiber kept up strong bars were attempted in this examination. All segments were openly kept up and given shear support other than at the expected second zone. Gotten closes delicate carbon steel strands with standard length of 60 mm (2.36 in.), clear distance across of 0.8 mm (0.03 in.), setting level of 75 and yield strength of 1100 MPa (159 500 psi) were used. A superplasticizer was used, and enough mixing time was allowed to pass on uniform mixing of concrete with no division. The speed of decay of the bar convincing sneak pinnacle of lethargy from the unmanaged changed to a totally broke region is lower for speaks with strands than that of shafts with no fibers

Chung-Chan Hung. et.al: This paper presents a three-dimensional constitutive model for the appraisal of HPFRCC structures. The fiber length was 38 mm with width of 0.038 mm. The constitutive relationship of HPFRCC materials under multi-center pressing factor conditions stay to be developed once the related groundwork results become available.

Z.L. Wang. et.al: This paper as an issue of first importance surveys the effect of fiber perspective degree (length-to-appraisal) on mechanical characteristics of steel fiber looked after concrete (SFRC). Appraisal uncovers that the viewpoint degree has an ideal flash for credits in every strong pack. The proportions of blows were gotten for SFRC containing 1.0%, 1.5% and 2.0% volume parts of strands, openly. A lower extra speed of shot and truly unassuming invasion openings in SFRC with higher point level of fibers are found in our calculations.

II. METHODOLOGY

MATERIAL STUDY - Preliminary tests for solid, fine aggregate, coarse complete is made and the properties out of materials are settled.

PLAN OF M60 GRADE CONCRETE - The mix strategy had the chance to pick the mix degree for M60 grade concrete IS 10262 : 2009.

EXPECTING AND TESTING OF CONCRETE SPECIMENS - Casting the occasion of M60 grade by adding fibers for full length of bars and strands simply in turned zones. Models are tried to ensure right mix degree and for assessment. By then the segments are attempted and results are isolated and standard concrete.

REESTABLISHING - The bar model and control models were kept in the shape for one day. After the significant length of 24 hours, they were segregated for later unmistakable attestation. By then the side plates of the bar were taken out. The fragment and collaborator model were managed in the water for 28 days easing. The shafts were taken from water to set them arranged for testing and allowed to dry for around 4 hours. The segments were then white washed and level and vertical lines were drawn at 40mm and 100mm stretch uninhibitedly.

RELATIONSHIP OF RESULTS - Conventional strong events of M60 evaluation of standard concrete is isolated and completely cross grouping strands and shaft with fibers fundamentally in turned zone. The aftereffects of the tests are portrayed. Diagrams are plotted for relationship of test results.

PRINCIPAL INVESTIGATION - Two unquestionable kinds of strands crimped steel fiber and got steel fiber are used in projecting FRC shaft self-governingly and joined as one. A degree of 70% got fibers and 30% crimped strands are gotten in this assessment. The volume a piece of fiber is fixed as 1.5% for the aggregate of the bars.

MATERIALS USED

PPC cement of 53 assessment is used for this starter assessment. The solid has a specific gravity of 3.1. Silica smoke is a bi-thing in the Silicon and ferrosilicon industry was used as a mineral admixture in solid mixes. It contains colossal degree of silicon-di-oxide (sio₂) which is about 90%of silica fume constituents. The fineness in silica smolder the degree that unequivocal surface territory is around 20000cm²/g. Silica fume fuses fine (<1um) particles and grows the security strength between solid paste and through and through by making the interfacial zone considerably more thick.

Sums are the essential fragments of concrete. It goes likely as reasonable space filler. IS 383 shows the necessities of aggregate. They are idle and are completely dispersed two classes i.e, fine and coarse out and out ward upon their size .The crushed stone is used as coarse aggregate and River sand enjoyed for fine aggregate

Coarse complete will join impeccable, hard, strong, thick, non-helpless and uncommon pieces of crushed stone. They will avoid pieces of crumbled stones, fragile, flaky, expanded particles, salt, destructive neutralizer, vegetable matter or other hurting materials. Crushed stone incomparable of most cutoff size 12.5mm are used as coarse aggregate. Likewise have express gravity worth of 2.7 and fineness modulus of around 7

Fine aggregates won't contain dust, ties, sensitive or flaky materials, mica or other damaging materials. Fine sums, having positive stomach settling agent silica reaction won't be used. The fineness modulus of fine all out will nor be under 2.0 nor more imperative than 3.5

Hooked end Fibres

The length of the fiber is 30 mm. The point of view extent of fiber is 48.4.The distance across of Hooked end strands is 0.62mm. The versatility of the fiber is 1100Mpa. Fig 4.1 shows the viewpoint on the trapped end fibers



Fig 1. Hooked End Fibres

CRIMPLED FIBRE

The length of the fiber is 38 mm. The point of view extent is 69.09). The estimation of crimped fiber 0.55mm. The flexibility is 600Mpa. The Material sort of crimped fiber is low carbon drawn level wire. The crimped fiber as shown in the figure.



Fig 2. Crimped Fibre

Minimized water open in the examination community is used for mixing and diminishing cement. CERA HYPER PLAST a sort of plasticer is used to assemble the usefulness of concrete. It is a high arrive at super plasticizing admixture. An estimations of 1 % by weight of clasp is used for all the mixes.Bars of Fe415 grade, 2 bars were 8mm width given at both top and base. 6mm distance across apex steel bars were used for stirrups.

Mix Proportioning

M60 grade strong mix was arranged by IS 10262-2009. Degree of concrete should be picked to make the most traditionalist use of available materials to convey concrete of required quality. The mix extent for projecting the model used is 1:1.2:2.2 and water solid extent 0.3. Volume parts of 1.5% are used strands. Furthermore 10 % of cement is displaced by silica fume intend to make HPC. for HFRC 70% Hooked fiber got together with 30 % crimped fiber were

consolidated as one in the important measure of fibers

Dimensions and Reinforcement Details

The column shape was set up by standard steel structure having cross region long. It is used for extending the bars with and without strands. Hence the size of the bar is of 80 x 120 x 2200mm. All the columns were projected with following help details. Four bars of 8mm dia are used as essential help 2 numbers at top and 2 numbers at base, 6mm dia stirrups are isolated at 100mm c/c to go about as shear support. The help nuances for the shaft models showed up in fig 4.3& 4.4

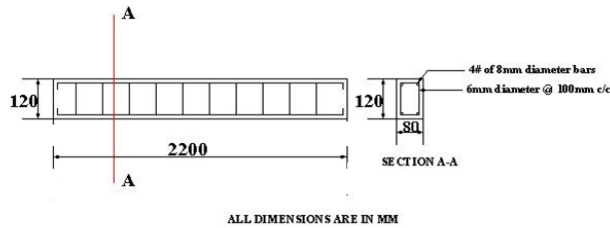


Fig 3 Reinforcement Details

Projecting of companion model - Hand mixing was embraced for profitable treatment of steel fiber . Sand and cement with silica rage were mixed dry and kept freely. By then coarse absolute was added and generally measure of water was sprinkled on the dry mix. To avoid the advancement of abnormalities by fragile sprinkling the fibers were heedlessly arranged in the strong mix. Two sorts of shafts one with totally cross variety strands and fibers simply in turned zone for flexural strength of plain concrete with and without fibers.

Projecting of Continuous Beams - The particular measures of materials for the model were weighted and kept freely before the mixing started. Machine mixing was gotten and the strong mix was placed in structure layer by layer and compacted well. Fig 5 shows the projecting of bars



Fig 4. Casting of beams

Test Set Up

The bars were put on the clear sponsorships to have a two territory steady column. A game-plan is showed up in fig 4.5. Two point loads was applied by inifers screw jack. The load was assessed by using dial estimates put picked stacking of the bar as shown in fig



Fig 5. Experimental beam set up 1

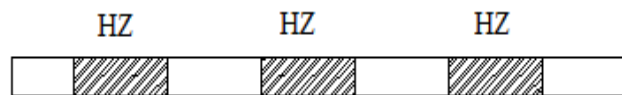


Fig 6. Experimental beam set up 2



Fig 7. Experimental set up

The basic lead of HPFRC shaft has been gathered in this endeavor. Two kinds of column specifically HPC and HPFRC emanates has been anticipated and attempted under monotonic stacking. Four common strong columns are given a job as first class concrete and in HPFRC transmits three bars were projected explicitly with totally cross variety strands and fiber simply in turned zone bar, and bar without fibers independently. In combination fiber developed strong bar, the model is melded with trapped end and crimped fibers in the mix degree of 70%-30% by volume at a total volume part of 1.5%. By then the eight columns are presented to cyclic stacking and eight bars are presented to monotonic stacking with the help of screw jack and the redirection is assessed by using deflectometer. In the wake of testing, various limits like energy digestion, total flexibility, first break weight and outrageous weight are differentiated and that of standard strong bar.

Behaviour of Continuous Beam Under Monotonic Loading

Experimental Investigation

Nine amounts of steady columns were anticipated and attempted. Five column is made with standard strong, Two is made with totally fiber and other two is made by adding fibers simply in turned zones. The shafts were doled out as follows for straightforward reference and presentations of the test result. Constant bar with normal concrete – CHPC

Steady columns with totally Hybrid fiber (1.5% Vf) – CFHC

Continous column with fibers in rotated zones just (1.5%Vf) – CFHC

Steady column with Hybrid fiber (70% Hooked end +30% Crimped) – CH+C

III. RESULTS AND DISCUSSION

Totally nine models have been gone after for their direct under monotonic stacking. To examine the effect of fibers. The test results are discussed as underneath. The different limits such weight passing on limit, strength, pliability, energy absorption limit, toughness list, etc have been resolved for all sbeams and showed up in Table.3.

**Table I
Test Results**

Parameter	BEAM									
	FHI		FH2		FHZI		FHZ2		C5	
	D1	D2	D1	D2	D1	D2	D1	D2	D1	D2
% of fibre (Vf)	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5
First crack load (kN)	75	75	57	57	42	42	60	60	39	39
Ultimate load (kN)	120	120	117	117	93	93	117	117	114	114
Stiffness (kN/mm)	66.67	50	50	50	50	66.67	66.67	66.67	40	50
Ductility factor	5	5.46	5	7	5.46	4.375	6.156	5.235	4.25	4.85
Energy Absorption(Kn.mm)	520	710	650	570	620	600	660	675	530	570
Toughness Index (I5)	6.65	7.5	7.5	7.33	7.71	7	6.85	7.21	4.76	5.26
Toughness Index (I10)	11.07	13.75	13.45	13.14	14.14	14.375	13.35	13.87	9.34	10.24

Load Carrying Capacity

A conclusive weight passing on constraints of the large number of columns are showed up in figure 3. The weight passing on restriction of FH shaft is higher than that of various bars. The chief break pile of FH shaft was higher than that of FHZ column. First break load extended with extension in fiber content. The column FH passes on most noteworthy limit pile of 120kN.

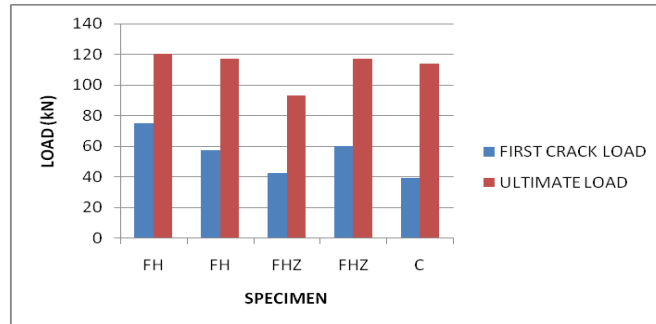


Fig.8 First crack load and Ultimate load for with and without fiber

Load deflection behaviour

From these assessments it is seen that the stack redirection twist is immediate up to the principle break load. Further development in load, caused various severs and the twist veered from linearity in to a non-straight region. From these comparisons the Beam-BI passes on most noteworthy weight passing on restriction of 120 kN.

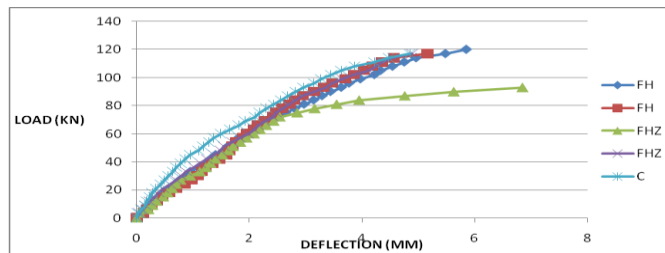


Fig 9. Comparison of Load deflection behavior for D1 of all the beams

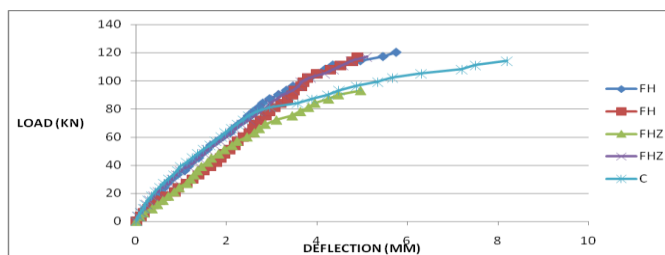


Fig 10. Comparison of Load deflection behavior for D2 of all the beams

STIFFNESS CHARACTERISTICS

Robustness is portrayed as the pile expected to cause unit aversion of the bar. A diversion is drawn at starting weight level. The grade of the diversion, thusly drawn, gave the immovability of the bar. The 1.5% volume part of fibers in HPFRC extended the immovability of the shafts. FH and FHZ emanates shows about higher solidness than CHPC. The diverse of immovability ascribes for all of the shafts are showed up in figure.5. The bars FH and FHZ are essentially relative in strength brand name with 66.67kN/mm.

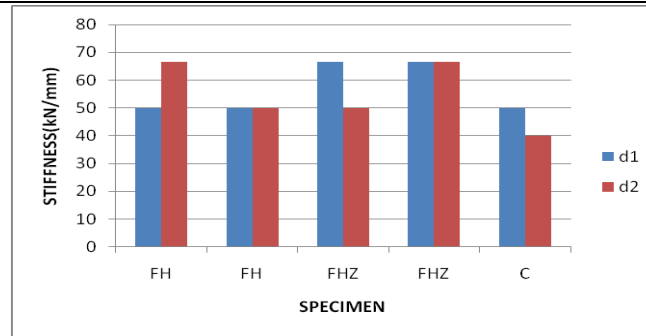


Fig 11. Comparison of Stiffness characteristics for all the beams

Cumulative Ductility

It is portrayed as the limit of a section goes through inelastic deformations past the yield mutilations without colossal setback in its stack passing on limit. The shaft FH has a constraint of 6.185. The flexibility of a flexural part can be procured from its stack redirection twist. The assortments of consolidated flexibility of the overall large number of four bars are showed up in figure

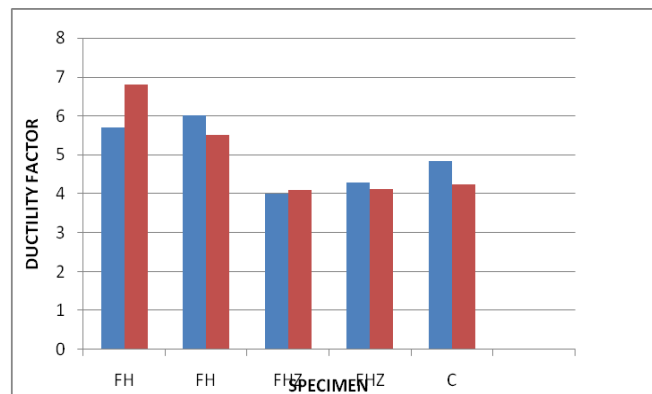


Fig 12. Comparison of Cumulative Ductility for all the beams

Energy Absorption Capacity

The domain under the store aversion twist tends to the energy maintenance breaking point of the all model. The total energy ingestion breaking point of FH bar was 710kNmm while that of FZH and CHPC emanates have the characteristics as 650kNmm, 570 kNmm independently. The total energy maintenance breaking point of FH (mutt) column was higher than that of various bars as exhibited in figure.7

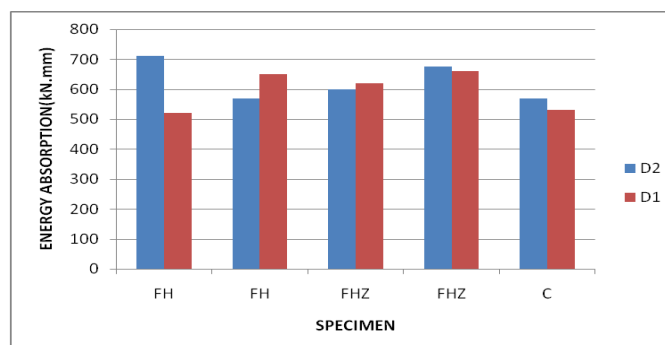


Fig 13. Comparison Energy absorption capacity for all the beams

Toughness Index(I5)

The domain under the stack aversion twist tends to the energy maintenance cutoff of the all model. The complete energy ingestion cutoff of FHZ column was 7.71 while that of FZH and CHPC emanates have the characteristics as 7.5 and 5.26 independently. The total Toughness index(I5) breaking point of FHZ (combination) shaft was higher than that of various columns as exhibited in figure 14

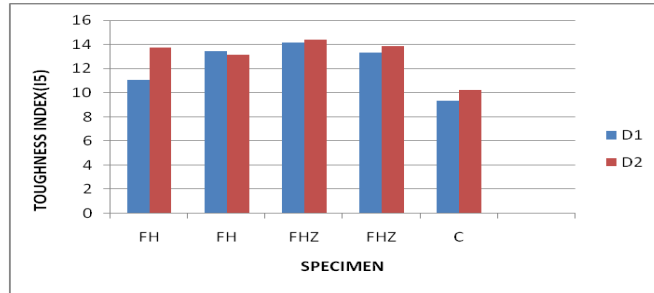


Fig 14. Comparison Toughness Index(I5) for all the beams

Toughness Index(I10)

The region under the load redirection twist tends to the energy ingestion cutoff of the all model. The all out energy ingestion cutoff of FHZ shaft was 14.37 while that of FZH and CHPC emanates have the characteristics as 13.75 and 10.24 separately. The total Toughness index(I5) cutoff of FHZ (cross variety) bar was higher than that of various columns as exhibited in figure 15.

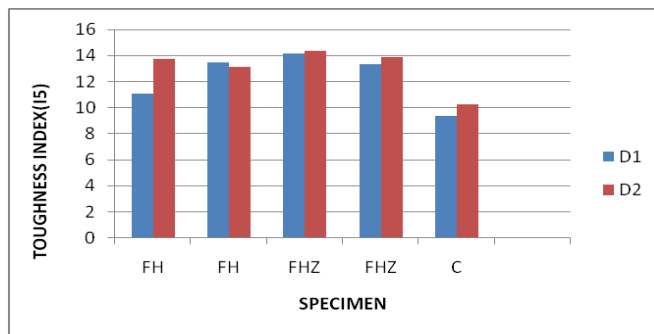


Fig 15. Comparison Toughness Index(I10) for all the beams

Behaviour and Mode of Failure

The stacking was extended in all of the models, the amount of splits appeared in the bar. Further growing the store, additional breaks outlined in the columns. It is seen that, models likewise upheld with steel fibers, a colossal number of better breaks outlined in the flexure zone and amplifying when the stack extended. The presence of steel fiber inside the column will go against the break improvement by forming a framework across the break for instance the steel strands go about as break catching material during early on period of stacking. The attempted instances of HPC and HPFRC transmits were showed up in figure.



Fig 16..Failure pattern for all the beams

CONCLUSION

Considering the preliminary outcomes the going with closes are drawn:

- Strengthening of HPC emanates using Steel strands achieved higher weight passing on limit. The rate development in outrageous weight changed from 23.51% to 88.22% for built up HPC emanates.
- The rate extension in aversion at outrageous stage changed from 1 % to 1.5% for HPC transmits strengthened with development of steel strands.
- Steel fiber braced bars slumped in flexural mode in a manner of speaking.
- The general direct of the restricted part models show incredible simultaneousness with discernments and data from the test tests. The failure arrangement of a developed strong shaft is shown very well using FEA and the mistake load expected is close to the failure load assessed during preliminary testing.

REFERENCE

1. Giuseppe Campione., “Maria Letizia Mangjvallano., “Fibrous reinforced concrete beams in flexure: Experimental investigation, analytical modelling and design considerations”., Vol.30., (2008). PP. 2970–2980.
2. Sudharsan N, Blessy Grant C J (2017), Structural Behaviour of Laced Reinforced Concrete Elements- A Review, International Journal of Current Engineering And Scientific Research, 4(12), 36-41.
3. Vidhya, K., & Kandasamy, S. (2014). Study on the flexural strength of coal ash brick masonry wall elements. Journal of Structural Engineering (India), 41(4), 410–419.
4. Aruna G, & Sukumar S “Stub columns tests of cold-formed steel built-up square sections with intermediate stiffeners”, Strength of Materials, Vol. 52, No. 2 (2020), pp. 281-290
5. P Jagadeesan, N Sudharsan, V Dhanalakshmi, (2020), Influence of Chicken Wire Mesh Wrapping on Strengthening of RC Beam, Journal of Xi'an University of Architecture & Technology, 12(4), 3327 – 3333.
6. Aruna G, Sukumar S & Karthika “Finite element analysis and design of cold-formed built-up closed columns with flange and web intermediate stiffeners”, Canadian Journal of Civil Engineering, 47 (2020), pp. 1175–1187
7. H. Akbarzadeh., A.A. Maghsoudi., “Experimental and analytical investigation of reinforced high strength concrete continuous beams strengthened with fiber reinforced polymer”., Vol.31., (2010). PP 1130–1147.
8. Aruna G, Sukumar S & Karthika “ Behaviour of cold-formed steel built-up closed columns composed by angle profiles”, Asian Journal of Civil Engineering , DOI/10.1007/s42107-019-00164-8 (2019)
9. N. Sudharsan, T. Palanisamy, S. C. Yaragal, (2018), Environmental sustainability of waste glass as a valuable construction material - A critical review. Ecology, Environment and Conservation, 24 pp. S331–S338
10. Samir A. Ashour ., Faisal F. Wafa, Mohmd I. Kamal., “Effect of the concrete compressive strength and tensile reinforcement ratio on the flexural behavior of fibrous concrete beams”., Vol. 22 (2000). PP 1145–1158.
11. Chung-Chan Hung., Shang-Heng Li., “Three-dimensional model for analysis of high performance fiber reinforced cement-based composites”., vol. 45 (2013). PP 1441–1447.
12. Joost C. Walraven ,” High performance fiber reinforced concrete: progress in knowledge and design codes”, Received: 11 July 2007 / Accepted: 11 June 2009 / Published online: 6 October 2009
13. S. Pant Avinash, R. Suresh Parekar, “ Steel fiber reinforced concrete beams undercombined torsion-bending-shear”, Journal of Civil Engineering (IEB), 38 (1) (2010) 31-38
14. Z.L. Wang., J. Wub, J.G. Wang., “Experimental and numerical analysis on effect of fibre aspect ratio on mechanical properties of SRFC”. Vol.24 (2010) 559–565.
15. P.Subba Rao I ,A.Venkateswara Rao., “A study on load -deflection behaviour of cracked concrete beam using fem: fracture mechanics approach”., Vol. 1 (2012) ISSN: 2278-0181
16. Antonio F. Barbosa and Gabriel O. Ribeiro., “Analysis of reinforced concrete structures using ansys nonlinear concrete model”., (1998). PP. 1-7.
17. C.X. Qian, P.Stroeven., “Development of hybrid polypropylene-steel fiber-reinforced concrete”., Cement and concrete., Vol.30. 2000., PP. 63-69.
18. Sudharsan, N., & Grant, B. C. J. (2018). Comparison of static response of laced reinforced concrete beams with conventional reinforced concrete beams by numerical investigations. International Journal of Civil Engineering and Technology, 9(8), 700–704
19. Vidhya, K., & Kandasamy, S. (2013). Study on properties of bricks manufactured using fly ash and pond ash. Pollution Research, 32(2), 405–409.
20. Manikandan P, Aruna G, Balaji S, Sukumar S & Sivakumar M, “Evaluation on effectiveness of cold-formed steel column with various types of edge stiffener”, Arabian Journal for Science and Engineering, DOI 10.1007/s13369-017-2571-6 (2017).
21. Vidhya, K., & Kandasamy, S. (2016). Experimental Investigations on the Properties of Coal-Ash Brick Units as Green Building Materials. International Journal of Coal Preparation and Utilization, 36(6), 318–325.